



# OCSD Water Reuse Facility

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## Project Overview

A new water treatment reuse facility must be designed to treat 20 MGD of wastewater. It will be located in a semi-arid inland region. This design entails pre-treatment, primary treatment, secondary treatment, and tertiary treatment processes. It also includes a solids-handling facility that treats sludge from the primary and secondary clarifiers. The treated water will be used for groundwater replenishment.

## Constraints and Parameters

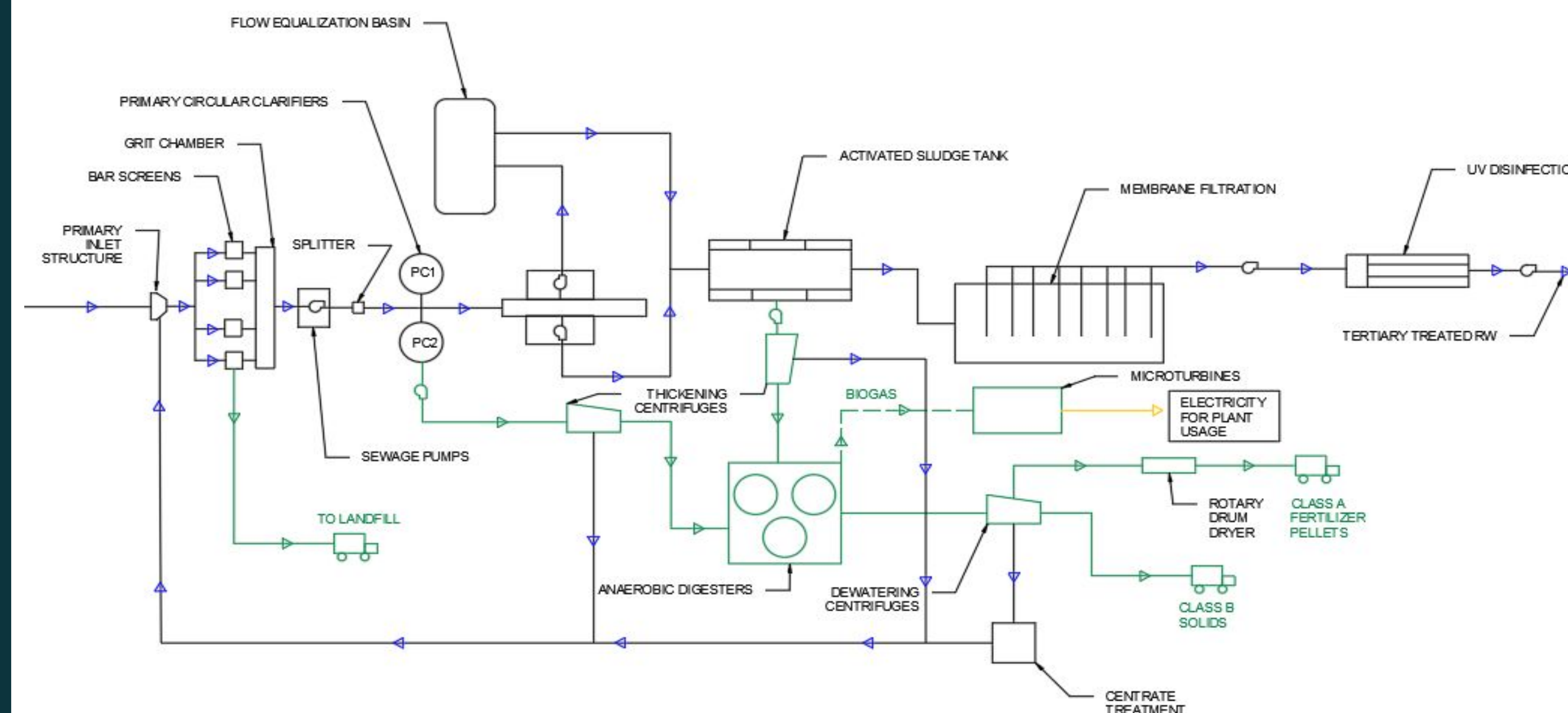
- ❖ Average daily flow of 20 MGD, with a peaking factor of 3 to accommodate extreme weather events.
- ❖ The treated effluent leaving must be in accordance to the California Code of Regulations, Title 22, to be used as recycled water. Additionally, the effluent must meet The Clean Water Act and National Pollutant Discharge Elimination System (NPDES) standards.
- ❖ Develop optimal solutions to prevent odors and pollution for public health and safety.

## Design Constituents

| Category         | Constituent                     | Influent | Discharge Standard | Units |
|------------------|---------------------------------|----------|--------------------|-------|
| Organic Matter   | Biochemical Oxygen Demand (BOD) | 300      | 30 <sup>1</sup>    | mg/L  |
|                  | Chemical Oxygen Demand (COD)    | 540      | 30 <sup>1</sup>    | mg/L  |
| Suspended Solids | Total Suspended Solids (TSS)    | 430      | 30 <sup>1</sup>    | mg/L  |
| Nutrients        | Ammonia-Nitrogen                | 40       | 2                  | mg/L  |

<sup>1</sup>: 30mg/L standard is based on a 30 day average

## Process Flow



## Pre-Treatment

| Mechanically Cleaned Bar Screen |             |                           |
|---------------------------------|-------------|---------------------------|
| Criteria                        | Value       | Units                     |
| Design Flow                     | 75708 (20)  | m <sup>3</sup> /day (MGD) |
| Head Loss                       | 300         | mm                        |
| Size Opening                    | 25          | mm                        |
| Moisture Content                | 70%         | —                         |
| Specific Weight                 | 7848        | N/m <sup>3</sup>          |
| Screen Capacity                 | 22          | L/1000 m <sup>3</sup>     |
| Volume Screened                 | 1666        | m <sup>3</sup> /day       |
| Dry Mass Screened               | 3921 (4.32) | kg/day (tons/day)         |

| Aerated Grit Chamber        |           |                      |
|-----------------------------|-----------|----------------------|
| Criteria                    | Value     | Units                |
| Peak Flow Rate              | 60        | MGD                  |
| Volume per Unit             | 8355.1    | ft <sup>3</sup>      |
| Depth                       | 12        | ft                   |
| Width                       | 18        | ft                   |
| Length                      | 38.7      | ft                   |
| Detention Time at Avg. Flow | 8.9       | minutes              |
| Air Supply                  | 386.8     | ft <sup>3</sup> /min |
| Grit Relative Density       | 2.65      | kg/m <sup>3</sup>    |
| Volume of Grit              | 200       | ft <sup>3</sup> /day |
| Mass of Grit                | 15 (33.1) | kg/day (lb/day)      |

## Primary Treatment

| Circular Sedimentation Tank                           |        |                                     |
|---|--------|-------------------------------------|
| Criteria  | Value  | Units                               |
| Hydraulic Retention Time                              | 2      | hours                               |
| Average Overflow Rate                                 | 40     | m <sup>3</sup> /m <sup>2</sup> -day |
| Peak Hourly Flow                                      | 100    | m <sup>3</sup> /m <sup>2</sup> -day |
| Total Surface Area Required at Average Flow Condition | 1893   | m <sup>2</sup>                      |
| Depth   | 4      | m                                   |
| Diameter  | 30     | m                                   |
| Bottom Slope  | 0.08   | mm/mm                               |
| Flight Speed  | 0.03   | rev/min                             |
| Area of Each Tank                                     | 730    | m <sup>2</sup>                      |
| Number of Tanks Needed                                | 3      | —                                   |
| Detention Time  | 2.6    | hours                               |
| TSS Concentration (60% Removal)                       | 172    | mg/L                                |
| BOD Concentration (34% Removal)                       | 198    | mg/L                                |
| Solids Concentration                                  | 6%     | —                                   |
| Sludge Specific Gravity                               | 1.03   | —                                   |
| Sludge Dry Mass                                       | 0.0195 | kg/day                              |
| Flow of Sludge  | 0.316  | L/day                               |

## Secondary Treatment

- ❖ Secondary treatment uses biological processes to remove organic matter, suspended solids, and nutrients, such as nitrogen and phosphorus.
- ❖ An activated sludge aeration tank will be utilized during this phase of treatment

## Tertiary Treatment

- ❖ Tertiary treatment removes the remaining inorganic compounds and organisms that may be harmful to public health such as bacteria, parasites, and viruses
- ❖ Tertiary treatment will consist of ultrafiltration membranes in conjunction with UV disinfection to ensure that the facility meets Title 22 regulations and NPDES standards.

## Initial Cost Estimate

| Criteria          | Cost         |
|-------------------|--------------|
| Pre-Treatment     | \$650,000    |
| Primary Treatment | \$27,400,000 |
| Total             | \$28,050,000 |

## Next Steps

- ❖ Secondary Treatment Design
- ❖ Tertiary Treatment Design
- ❖ Solids Handling
- ❖ Total Cost Analysis

## Contact Information

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